GENERAL NOTES:

1. BUILDING CODE : 2003 INTERNATIONAL BUILDING CODE W/ HOUSTON AMENDMENTS.

2. CODES AND STANDARDS: LATEST U.N.O.
REINFORCED CONCRETE: ACI 318-02-AMERICAN CONCRETE INSTITUTE
STRUCTURAL STEEL: AISC -AMERICAN INSTITUTE OF STEEL CONSTRUCTION

3. GENERAL CONTRACTOR SHALL COORDINATE ALL DIMENSIONS AND SHALL NOTIFY ARCHITECT OF ANY DISCREPANCIES WHICH MAY EXISTS.

4. ALL OPENING THROUGH STRUCTURAL MEMBERS SHALL BE AS SHOWN ON STRUCTURAL DRAWINGS. GENERAL CONTRACTOR SHALL COORDINATE ALL OPENING REQUIREMENTS FOR MECHANICAL CONTRACTOR, ANY DEVIATION FROM THE OPENING SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE BROUGHT TO THE ENGINEER'S ATTENTION FOR HIS APPROVAL.

5. DESIGN LOADS
DEAD LOAD
SELF WEIGHT OF MEMBERS
SUPERIMPOSED DEAD LOAD
FINISHES ..... AS INDICATED ON ARCHITECTURAL DRAWINGS
LIVE LOAD
ROOF ..... 20 PSF
WIND LOAD 110 MPH (3 SECOND GUST) ; EXPOSURE 'B'

FOUNDATION IS DESIGNED BASED ON RECOMMENDATIONS OF GEOTECHNICAL REPORT PREPARED BY ASSOCIATED TESTING LABORATORIES, INC. REPORT NO. GOS-394 DATED AUGUST 23,2005

CONCRETE:

MATERIAL:
1. CONCRETE SHALL BE NORMAL WEIGHT CONCRETE WITH MINIMUM f'c AS INDICATED BELOW.

A. PIERS, FOOTINGS ..... f'c=3000 PSI AT 28 DAYS.
B. PIER CAPS, GRADE BEAM, SLAB ON GRADE ....f'c=4000 PSI AT 28 DAYS.

2. REINFORCING STEEL SHALL CONFORM TO FOLLOWING GRADE.
REINFORCING BARS ASTM A-615 GRADE 60 EXCEPT NOTED
WELDED WIRE FERRIC ASTM A-185
REINFORCING BARS WELDED TO STEEL SHALL BE WELDABLE GRADE 40

3. DETAILING OF REINFORCING BARS AND ACCESSORIES SHALL BE IN ACCORDANCE WITH ACI PUBLICATION 315, LATEST EDITION.

4. ALL MIXING TRANSPORTING, PLACING, CURING OF CONCRETE SHALL BE ACCORDANCE WITH RECOMMENDATION OF AMERICAN CONCRETE INSTITUTE.

5. ALL MIX DESIGN SHALL BE SUBJECT TO ENGINEER'S APPROVAL.

6. ALL REINFORCING BARS SHALL HAVE CLASS B LAP SPLICE PER ACI-318-02 SECTION 12.15 UNO ON DRAWINGS.

7. ALL LAPS FOR WELDED WIRE MESH SHALL BE ONE MESH PLUS THREE INCH AT ALL LAPS.

8. PROVIDE 3"x6"x20 GAGE SHEET METAL BAR CHAIR AT 4'-0" O.C. EACH WAY FOR ALL TOP REINFORCING FOR SLAB ON GRADE.

9. ALL CONSTRUCTION JOINTS SHALL BE SUBJECT TO ENGINEERS APPROVAL.

10. GRADE BEAM SHALL BE POURED ON CORRUGATED FIBER CARTON FORMS WHEN INDICATED ON DRAWINGS.

11. GRADE BEAM CONSTRUCTION JOINT SHALL BE LOCATED AT CENTER OF THE SPAN. ALL CONSTRUCTION JOINTS SHALL HAVE A SHEAR KEY AND ADDITIONAL REINFORCING AS SHOWN ON DRAWINGS. ALL OPENING THROUGH GRADE BEAMS SHALL BE AS INDICATED ON DRAWINGS.

12. FORMS SHALL NOT BE REMOVED UNTIL FOLLOWING STRENGTH IS REACHED. THE STRENGTH SHOWN BELOW IS THE PERCENTAGE OF 28 DAY COMPRESSIVE STRENGTH f'c.

FLOOR SHORES	85 PERCENT
SLAB, JOIST, BEAMS AND GIRDER BOTTOM	70 PERCENT
WALL, COLUMN, BEAM SIDE	40 PERCENT

13. ALL WALL SHALL BE NOT BE BACKFILLED UNTIL THE WALL SUPPORTING MEMBERS HAVE ATTAINED MINIMUM OF 70 PERCENT OF THEIR 28 DAY SPECIFIED STRENGTH.

14. ALL POURED IN PLACE CONCRETE MEMBERS SHALL HAVE FOLLOWING CLEAR COVER TO REINFORCING.

FOOTING AND PIERS		3 INCH
GRADE BEAMS BOTTOM		3 INCH
	SIDE POURED AGAINST SOIL	3 INCH
	SIDE FORMED	2 INCH
	TOP	1 1/2 INCH
WALLS	SIDE EXPOSED TO EARTH	2 INCH
	ALL OTHER	3/4 INCH
SLAB ON GRADE		1 INCH (TOP)

TESTING AND INSPECTION NOTES

1. ALL STRUCTURAL STEEL WORK SHALL BE INSPECTED BY A TESTING AND INSPECTION AGENCY HIRED BY THE OWNER.

2. ALL WELDING SHALL BE PERFORMED BY A CERTIFIED WELDER AND INSPECTED BY THE TESTING AND INSPECTION AGENCY.

3. ALL BOLTS SHALL BE VISUALLY INSPECTED AND 20% (MINIMUM TWO BOLTS PER CONNECTION) SHALL BE TESTED FOR THEIR FULL PRESCRIBED PRETENSION.

4. ALL FULL PENETRATION WELD SHALL BE TESTED BY ULTRASONIC TEST.

5. CONCRETE STRENGTH STEST SHALL BE PER REQUIREMENTS OF AMERICAN CONCRETE INSTITUTE.

STRUCTURAL STEEL

1. ALL STEEL MEMBERS UNO ON DRAWINGS	ASTM A 572, GRADE 50 ASTM A 36
BASE PLATES, MISC. STEEL PLATES UPTO 4 INCH THICK	ASTM A 572, GRADE 50
CHANNELS AND ANGLES	ASTM A 36
HOT FORM TUBULAR SECTION	ASTM A 501
COLD FORMED TUBULAR SECTIONS	ASTM A 500
CONNECTION BOLTS	ASTM A 325
ANCHOR BOLTS	ASTM A 307
HARDENED WASHER	ASTM F 436

2. ROOF DECK SHALL HAVE MINIMUM 2 INCH LAPS AND SHALL BE FASTENED AS INDICATED ON PLAN NOTES.

3. GROUT UNDER BASE PLATES SHALL BE NON SHRINK WITH MINIMUM COMPRESSIVE STRENGTH OF 5000 psi AT 28 DAYS.

4. STRUCTURAL STEEL FABRICATION AND ERECTION SHALL CONFORM TO THE AISC "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS" AND THE AISC "CODE OF STANDARDS PRACTICE FOR STEEL BUILDING AND BRIDGES."

5. SPLICING OF STRUCTURAL STEEL MEMBERS IS PROHIBITED WITHOUT PRIOR WRITTEN APPROVAL OF STRUCTURAL ENGINEER FOR EACH SPECIFIC CASE.

6. ALL EXPOSED STEEL SHALL BE HOT DIP GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH ASTM A 123. FIELD WELDS AT CONNECTIONS SHALL BE PAINTED WITH GALVUME PAINT.

7. ALL CONNECTIONS NOT DETAILED ON THE DRAWINGS SHALL BE DESIGNED AND DETAILED BY THE FABRICATOR AND SHALL BE SUBMITTED PRIOR TO FABRICATION TO THE STRUCTURAL ENGINEER FOR REVIEW.

8. BEAM TO BEAM AND BEAM TO COLUMN CONNECTION SHALL DEVELOP THE END REACTION NOTED ON THE DRAWING. IF NO END REACTIONS ARE INDICATED ON DRAWINGS CONNECTION SHALL BE DESIGNED FOR VALUE OBTAINED FROM LOAD CARRYING CAPACITY OF THE BEAM ON THE BASIS OF EQUIVALENT UNIFORM LOAD GIVEN IN THE PART OF 2 OF AISC MANUAL OF STEEL CONSTRUCTION, 1989 (9th EDITION).

9. MINIMUM END REACTION FOR NON COMPOSITE BEAMS SHALL BE 8 KIPS UNLESS NOTED ON DRAWING AND MINIMUM END REACTION FOR COMPOSITE BEAMS SHALL BE 12 KIPS UNLESS NOTED ON DRAWINGS.

10. ALL WELDING SHALL CONFORM TO THE STANDARD OF AMERICAN WELDING SOCIETY. ALL WELDS NOT SHOWN SHALL BE AWS MINIMUM. WELDING ELECTRODES FOR ALL WELDS SHALL CONFORM TO ANSI/AWS D1.1 (LATEST EDITION.

11. FOR ALL HIGH STRENGTH BOLTS A HARDENED WASHER SHALL BE PROVIDED ON NUT SIDE OF BOLT FOR TORQUING AS REQUIRED.

12. STEEL BEAMS BEARING ON MASONRY SHALL HAVE MINIMUM 8 INCH BEARING AND ANCHORS AS SHOWN ON DRAWINGS. THE CELLS UNDER BEAM BEARING SHALL GROUTED FOR FULL HEIGHT OF WALL FROM FOOTING TO BEAM BEARING.

13. CONTRACTOR SHALL PROVIDE SUPPORT FOR METAL DECK AT ALL SLAB EDGE AND AT COLUMN CLOSURE.

14. TEMPORARY SHORING OR BRACING DURING THE CONSTRUCTION PHASE BEFORE COMPLETION OF CONNECTION OF FLOOR SLAB OR ROOF DECK IS THE RESPONSIBILITY OF THE CONTRACTOR.

JOISTS

1. OPEN WEB JOIST SHALL CONFORM TO STANDARDS OF THE STEEL JOIST INSTITUTE.

2. ERECT JOIST PER RECOMMENDATIONS OF STEEL JOIST INSTITUTE AND PROVIDE BRIDGING DURING CONSTRUCTION AS REQUIRED BY STEEL JOIST INSTITUTE.

3. JOIST SHALL BE WELDED TO SUPPORTING STEEL BEAM OR GIRDERS FOR A LENGTH OF TWO 2 INCH - 3/16 IN WELD ON EACH SIDE OF SEAT.

4. WHERE COLUMNS ARE NOT FRAMED IN AT LEAST TWO DIRECTION BY STEEL BEAMS, BAR JOIST SHALL BE FIELD BOLTED IN BOTH DIRECTIONS FOR LATERAL STABILITY DURING CONSTRUCTION.

JOIST GIRDER

1. ALL JOIST GIRDER SHALL BE DESIGNED FOR LOAD INDICATED ON DRAWINGS. LOADS INDICATED ON DRAWINGS DOES NOT INCLUDE SELF WEIGHT OF JOIST GIRDER.

2. JOIST GIRDER DESIGN INCLUDING STABILITY REQUIREMENTS WILL BE SUBJECT TO ENGINEER'S APPROVAL.

LIGHT GAUGE STEEL FRAMING

1. ALL LIGHT GAUGE MEMBER PHYSICAL PROPERTIES AND ALLOWABLE LOAD CAPACITIES SHALL BE DEVELOPED IN ACCORDANCE WITH AISI SPECIFICATIONS FOR DESIGN OF COLD FORMED STRUCTURAL STEEL MEMBERS "LATEST EDITION."

2. MATERIALS
COLD - FORMED LIGHT GAUGE SHAPE ASTM A570 OR A 607 Fy = 55 ksi

3. ALL SCREWS SHALL HAVE A CORROSION RESISTANT FINISH.

4. MINIMUM EDGE DISTANCE FOR ALL SCREWS SHALL BE 1/2 INCH.

5. WELDING OF LIGHT GAUGE STEEL MEMBERS SHALL CONFORM TO THE REQUIREMENTS OF AWS D-1.1, AWS D-1.3.

6. SPLICING OF ANY MEMBER IS PROHIBITED.

7. ALL SCREWS FOR DECK AND WALLL PANEL CONNECTION SHALL BE #12-14 SELF DRILLING, SELF - TAPPING. SCREW HEADS SHALL BE HEX HEAD WASHER STYLE WITH OR WITHOUT SLOT UNO. ALL SCREWS SHALL HAVE NEOPRENE WASHERS.

8. All LIGHT GAGE MEMBERS SHALL BE CONNECTED USING 1/2 INCH. DIA BOLTS WITH MINIMUM 2 BOLTS PER CONNECTION.

9. ALL STEEL FRAMING COMPONENT SHALL BE CUT SQUARE FOR ATTACHMENT TO PERPENDICULAR MEMBERS OR AS REQUIRED FOR A FULL ANGULAR FIT AGAINST ABUTTING MEMBERS. ALL STUDS SHALL HAVE FULL BEARING AGAINST THE INSIDE WEB OF THE TRACK MEMBER. ALL MEMBERS SHALL BE SECURED IN PLACE UNTIL PROPERLY ATTACHED.

10. CONTRACTOR SHALL SUBMIT SEALED SHOP DRAWINGS FOR COMPONENT OF STRUCTURE WITH ALL MEMBERS SIZES, MATERIAL GRADE, CONNECTION AND BRACING DETAILS.

11. THE SHOP DRAWINGS SHALL INCLUDE SEALED CALCULATIONS PREPARED BY A REGISTERED PROFESSIONAL ENGINEER.

12. ALL MEMBERS SHALL BE DESIGNED FOR LOAD INDICATED ON CONTRACT DOCUMENTS.

WALLS

A. PARTITION WALLS

1. ALL WALLS SHAL HAVE LATERAL BRACING (HORIZONTAL) AT 48" O.C
2. PROVIDE STRAP X BACE FOR 5 LONG SEGMENTS OF A WALL IN ALL WALLS.
3. ALL STUDS SHALL BE PLUMBED, ALIGNED AND SECURELY ANCHORED TO THE TOP FLANGE OR WEB OF BOTH UPPER AND LOWER TRACK. PROVIDE FASTNER AT 6 INCH ON CENTER AND TWO FASTNER 1 ES OF WEB NEAR STUD UNLESS NOTHES OTHERWISE ON DRAWING.
4. ALL TRACKS SHALL BE SPLICED TO DEVELPO FULL STRENGTH. BOTTOM TRACKS SHALL BE SECURELY ATTACHED CONCRETE FLOOR.

B. SHEAR WALLS

1. PROVIDE SHEATHING AND GASTNERS AS INDICATED ON DRAWING.

2. ALL SHEAR WALL BOUNDAEY MEMBERS SHALL BE DOUBLE STUDS BACK TO BACK. CONNECT BACK TO BACK STUDS WITH #12-14 SCREWS AT 6" O.C.

3. CONNECT BOUNDRY MEMBERS TO FLOOR WITH HOLDOWN MEMBER AS INDICATED ON DRAWING.

4. PROVIDE BLOCKING AT ALL SHEATHING JOINTS. BLOCKING SHALL BE STUD MEMBER FOR FORST TWO SPACES OF STUDS, ALL OTHER BLOCKING CAN BE STRAP BLOCKING.

5. ALL STUDS SHALL BE CONNECTED TO TRACKS AS INDICATED BELOW.
AT BOTOM STUDS SHALL BE CONNECTED TO EACH FLANGE OF TRACK.
TRACK SHALL BE CONNECTED TO CONCRETE OR DECK WITH 0.14 INCH. SHANK DIAMETER POWDER ACTUATED FASTNERS MIINIMUM 1 1/2 INCH INTO CONCRETE.

6. ALL TRACKS SHALL BE SPLICED TO DEVELOPE FULL STRENGTH.

7. PROVIDE LATERAL BRACING (HORIZONTAL) AT 48" O.C

APPROVED
FOR BUILDING PERMIT ONLY
CITY OF HOUSTON
CODE ENFORCEMENT DIVISION
The owner is responsible for compliance with the Building Code. Such approved plans and specifications shall not be changed, modified or altered without authorization from the building official, and all work shall be done in accordance with the approved plans.
NO. 358

Community Family Centers
Centros Familiares de la Comunidad

John P. McGovern, M.D.
Community Sports & Recreation Building
7318 AVENUE F
HOUSTON, TEXAS 77011

THAT'S ARCHITECTURE
2402 COMMONWEALTH #100, HOUSTON, TX 77006
TELEPHONE 713/222-1406 FAX 713/222-1506

STATE OF TEXAS
R.D.PATEL
85114
LICENSED PROFESSIONAL ENGINEER
8/7/06

ISSUES & REVISIONS		
1	8/7/06	ISSUED FOR PERMIT

SHEET TITLE
GENERAL NOTES
SECTIONS

PROJECT NO. 2001-30
SCALE AS NOTED

SHEET NUMBER
S 0.0